

Bridges and Structural Safety

A brief introduction to principles and challenges

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Structural Safety
- a principle demand



Structural Safety...

...a principle demand (by law):

- **Plan - og bygningsloven: § 29-5**
*(Konstruksjonen) skal prosjekteres og utføres slik at det ferdige tiltaket oppfyller **krav til sikkerhet**, helse, miljø, energi og bærekraftighet, og slik at vern av liv og materielle verdier ivaretas.*
- **Forskrift om krav til byggverk (TEK): § 7-33**
*Byggverk skal utformes og dimensjoneres slik at de har tilfredsstillende **sikkerhet mot brudd** for de laster som kan oppstå under den forutsatte bruk. (...) Dersom forutsetningene for bruk endres, skal byggverkets sikkerhet vurderes på nytt.*

Structural Safety for New Structures

Attaining Structural Safety

...by following a combination of:

- **Design/Dimensioning for safety**
 - Structural capacity is controlled such that it is (most probably) larger than the demand.
 - The comparison is done by a so called limit state G , i.e. $G = \text{Capacity} - \text{Demand}$, such that $G < 0$ is defined as *failure* and the *failure probability* is correspondingly $P(G < 0)$.
 - Safety requirement might be formulated in terms of a target failure probability.
- **Quality Control**
 - Assuring that planning, design, execution and use is according to specifications and no errors are present.

Requirements on Safety

How safe is safe enough?

In the Eurocodes *appropriate* level of reliability is dependent on:

- the possible cause and /or **mode** of attaining a limit state;
- the possible **consequences** of failure in terms of risk to life, injury, potential economical losses;
- public **aversion** to failure;
- the **expense** and procedures necessary to **reduce** the risk of failure.

5

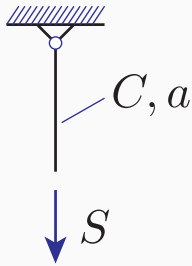
Safety requirement

Reliability Class	Minimum values for β	
	1 year reference period	50 years reference period
RC3	5,2	4,3
RC2	4,7	3,8
RC1	4,2	3,3

Figure 1: Reliability requirements as stated in EN 1990:2002

6

Reliability based design - a simple example



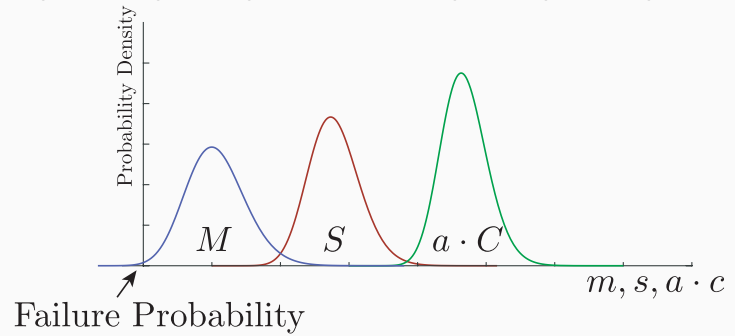
	μ	σ
Capacity C [kN/mm^2]	1	0.10
Load S [kN]	1	0.34

C, S Normal distributed.

$$\text{Failure} = \{a \cdot C - S < 0\}$$

Failure Probability:

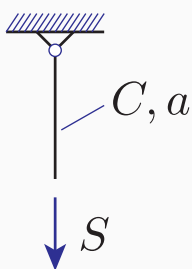
$$P(\text{Failure}) = P(a \cdot C - S < 0) = P(M < 0)$$



$$P(M < 0) = \Phi\left(\frac{0 - \mu_M}{\sigma_M}\right) = \Phi\left(\frac{-a \cdot \mu_C + \mu_S}{\sqrt{a^2 \sigma_C^2 + \sigma_S^2}}\right) = \Phi(-\beta)$$

7

Reliability based design - a simple example



	μ	σ
Capacity C [kN/mm^2]	1	0.10
Load S [kN]	1	0.34

C, S Normal distributed.

Reliability based design with target reliability
 $\beta_t = 3.8$:

$$a \text{ such that } \beta_{req} = 3.8 \leq \beta = \frac{a \cdot \mu_C - \mu_S}{\sqrt{a^2 \sigma_C^2 + \sigma_S^2}}$$

In this example equality is obtained for
 $a = 2.615 \text{ mm}^2$.

8

- A design can be identified that **corresponds** to a specified reliability requirement.
- The most simple form of reliability problem was considered here, but in practice it is often much **more complex**.
- The achieved reliability is **conditional on utilised knowledge** - the reliability based design solution is also conditional on knowledge!
- Reliability is always dependent on specified **reference time**.

Practical Design

Practical Design

Recapturing the Example from above, Safety/Reliability is evaluated based on a limit state function:

$$a \cdot C - S < 0$$

which required (cumbersome) reliability analysis.

In practical design, Safety/Reliability is evaluated based on *Design Equations* that are related to the limit state:

$$a \cdot \frac{c_k}{\gamma_C} - \gamma_S \cdot s_k < 0$$

Here, the random variables are replaced by a combination of so-called characteristic values (c_k, s_k) and safety factors (γ_C, γ_S). No reliability analysis is necessary.

10

Practical Design

According to the Eurocode Design Equations are generally used. Corresponding information is provided by the standard, i.e.:

- safety factors for different structural materials and loads,
- characteristic values for material strength, load intensity,
- modification factors to account for temporal, spatial and/or combination effects.

The information is generalised and valid for the design of new structures!

11

In particular, the following simplifications should be observed:

- spatial variability of material strength is assumed to be small and neglected,
- load process ergodic, resistance constant (once it is realised),
- safety evaluated at single cross sections individually.

- practical design based on generalized information and subject to simplifications,
- reliability requirement not to be seen as absolute by relative to the generalized and simplified representation.
- **However**, the approach works: Structures are perceived safe and (sufficiently) efficient.

Structural Safety of Existing Structures



Reassessing Structural Safety

- **As part of maintenance**

- The safe use of the structure has to be guaranteed during the service life.
- No reassessment necessary if (1) resistance can be assumed to be constant and (2) load ergodic.
- Reassessment necessary when doubts about (1) and (2).

- **Service life extension**

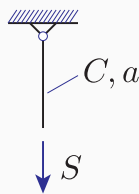
- The safe use of the structure has to be confirmed after the projected service life has passed.

- **Change in use**

- Change in use implies different loading than projected and requires reassessment.

15

Reliability reassessment - a simple example (cont.)



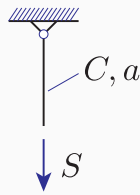
	Type	μ	σ
Capacity C [kN/mm^2]	Logn	1	0.10
Load S [kN]	Gumbel	1	0.34
Corrosion loss R [mm/a]	Logn	0.1	0.05

$$a = 2.6 \text{ mm}$$

$$\text{Failure} = \{(a - R \cdot t) \cdot C - S < 0\}$$

- The limit state has to be extended in order to represent the deterioration mechanism.
- The reliability becomes time dependent (and the decay of reliability can be predicted based on the model assumptions).
- All variables can be updated based on new information. E.g. for R direct or indirect information might be utilized.
- The consideration of information has a direct effect on the computed reliability!

Reliability reassessment - a simple example (cont.)



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Capacity C [kN/mm^2]	Logn	1	0.10
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$$a = 2.6 \text{ mm}$$

$$\text{Failure} = \{(a - R \cdot t) \cdot C - S < 0\}$$

Challenges

- The a-priori model for deterioration is difficult to formulate due to large uncertainties.
- Deterioration processes often add a spatial dimension to the problem.
- Measurements/observations provide often only indirect information, updating requires likelihoods and these are difficult to specify.

17

Reliability reassessment - Bridge Challenges



Bridge Challenges

- The mechanical system is far **more complex** and many simplifications taken during the design are not longer valid.
- Deterioration affects the strength **locally** - global measurements are not very informative; local measurements are likely misplaced.
- The computed **reliability** (as relative to the model representation) is **difficult to compare** with absolute reliability requirements.
- Mitigation measures are costly and often hard to implement.

18

Conclusion

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- The principles of safety assessment of new and existing structures was briefly addressed.
- The level of detail was correspondingly low but sufficient to highlight the main challenges of safety assessment.
- In practical assessment situations aspects that are not directly related to safety are often addressed as a proxy for the overall “health” of the structure.
- The presentations at the workshop will deepen the discussion to a much more informative level of detail from the perspective of research, infrastructure owner and technical consultant.

Questions?